

FLOOD CONTROL IN THE SEKANAK RIVER OF PALEMBANG CITY

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Abstract. Floods during the rainy season has become routine events in several cities in Indonesia. Various reasons to trigger the occurrence of flooding, among other drainage network system capacity is decreased, increasing water flow, or a combination of both. The capacity of the drainage channel has been calculated based on the design criteria to accommodate the flow of water occurs so that the area is not experiencing inundation or flooding. The reduced system capacity due to, among others, many precipitates, physical damage or their network systems and illegal buildings on the system network. While the cause of the discharge increases, among others, high rainfall out of habit, changes in land-use, environmental damage to the basin in a region. Cases like mentioned above also occur in Sekanak Sub-basin, so it is necessary to study the drainage network performance evaluation system based on the concept of sustainable drainage based on community participation. Good and bad, high and low of the drainage network system performance is largely determined by community participation in management, especially with the lack or absence of funds from Palembang city government for the management of drainage network system. Drainage system performance can be evaluated from the technical aspects as well as non-technical. One technical aspect is the frequency analysis to look at the picture unit hydrograph. Analytical results from this study may be that the maximum flow of 25 m³/sec at peak hours at the time of 4.8 hours and then slowly starting to go down at a time to 24 hours. The others aspect is Social construction problems can be solved by Public Participation approach, with the Government acting on the principle of fair, development activities carried out with transparency and attention to the needs of the community. Social problems occurring both at the pre-construction stage (land acquisition and resettlement), construction (procurement and mobilization of labor, mobilization of heavy equipment and construction materials and equipment operations) and post-construction can be anticipated by social engineering forming between Other identification of community character and institutions, socialization, public consultation, community gathering and community

Keywords: flood control, drainage network system, unit hydrograph, public participation

I. INTRODUCTION

The flood disaster became a regular phenomenon in the rainy season which is spread in different basin in most parts of Indonesia. Total incidence of flooding in the rainy season over the last 3 years as well as the increasing number of human casualties and loss of property and facilities from public or social, transport infrastructure and infrastructure for agriculture or irrigation. In addition to the problem of precipitation as factors, the incidence of disasters can not be separated from environmental damages to ecosystems that occur in the basin and poor management of water resources. Their land damage leading to increased surface run-off coefficient greater. The area upstream basin is an area of a particle will be increasingly vulnerable to drought, precisely the opposite downstream areas prone to

flooding. Flooding is a flow that caused economic losses or even cause loss of life.

Flow can occur because of the outburst in the area in the right or left of the river due to river channel does not have enough capacity for the flow rates through. Flooding is not only experienced by urban area located in the lowlands, even experienced area located in the highlands. Flooding or inundation in a region occurs when the system that serves to accommodate the inundation was not able to accommodate the discharge flow, it is the result of three possibilities occur: the capacity of the system to decrease the flow rate of water increases, or a combination of both. Understanding the system here is a system of drainage network in a region.

While the drainage system can be generally defined

Table 2. Short-term data maximum rainfall

Years	Duration 60 minute	Years	Duration 60 minute
1991	73.50	2003	71.50
1992	80.10	2004	80.00
1993	55.70	2005	91.90
1994	48.50	2006	60.90
1995	60.00	2007	65.00
1996	47.20	2008	86.00
1997	91.80	2009	79.50
1998	52.30	2010	90.00
1999	73.50	2011	80.00
2000	70.40	2012	86.93
2001	77.00	2013	77.63
2002	60.90		

Source: BMKG Kenten, 2014

Table 3 Rainfall intensity

Years	Intensity (mm/hours)	Years	Intensity (mm/hours)
1991	73.50	2003	71.50
1992	80.10	2004	80.00
1993	55.70	2005	91.90
1994	48.50	2006	60.90
1995	60.00	2007	65.00
1996	47.20	2008	86.00
1997	91.80	2009	79.50
1998	52.30	2010	90.00
1999	73.50	2011	80.00
2000	70.40	2012	86.93
2001	77.00	2013	77.63
2002	60.90		

Source: BMKG Kenten, 2014

C. Frequency Analysis

Before analyzing rainfall distribution, first determine the parameters of existing statistics. Then calculate the total amount, the amount of data (n), the maximum data Ri average, standard deviation (S), Coeffisien of Variation (Cv), Coeffisien of Skewness (Cs), and Coeffisien of kurtosis (Ck).

The result of the calculation as follows:

- Total amount = 1660.26 mm / hour
- Total rainfall data, n = 23
- On average Ri = 72.18 mm / h
- The standard deviation (S) = 13.66
- Coeffisien of Variation (Cv) = 0.19
- Coeffisien of Skewness (Cs) = -0.34
- Coeffisien of kurtosis (Ck) = -0.90

D. Rainfall Intensity

The intensity of rainfall that is used is the rainfall intensity data from the calculation of the Normal Distribution.

Period re-elected for further calculations that the return period of 2 years.

Table 4 Rainfall intensity

R (Year)	X _T (mm/hours)
2	72.18
5	83.66
10	89.67
20	94.59
50	100.19
100	104.01

Source: analysis result, 2016

E. Run-off Coefficient

Table 5 Run-off Coefficient

Region	Land use	C
Urban	Rural Settlement Region:	
	- Low density	0,25-0,40
	- Middle density	0,40-0,70
	- High density	0,70-0,80
	- With wells impregnation	0,20-0,30
	Trade zzone	0,90-0,95
	Industry region	0,80-0,90
	Parks, green lanes, gardens,	0,20-0,30
	etc.	0,40-0,60
	Hills, slopes < 20 percent	0,50-0,60
Rural	Canyons region, the slope of > 20 percent	0,25-0,35
	Land with terracing Rice fields	0,45-0,55

Source: PUBMSDA, 2014

Run-off coefficient reflects the state of the surface flow area. Drainage coefficient, C is the ratio of the volume of water that reached the mouth of the river basin with the volume of water that fell on the watershed. Value for drainage coefficient, C can be seen in Table 5.

Data obtained from Bappeda Kota Palembang, extensive land use for residential areas are:

Size high density = 7.09 km²

Extensive catchment area = 7.37 km²

Comprehensive trade area = 4.73 km²

Based on the flow coefficient table 5 for residential areas with a high density area and retrieved 0.70 to 0.20 wide catchment areas taken as well as to extensive trade area taken 0.90.

Then the value Cw: runoff coefficient values obtained, Cw = 0:56 and in the calculation taken Cw = 0.60.

F. Discharge Curve

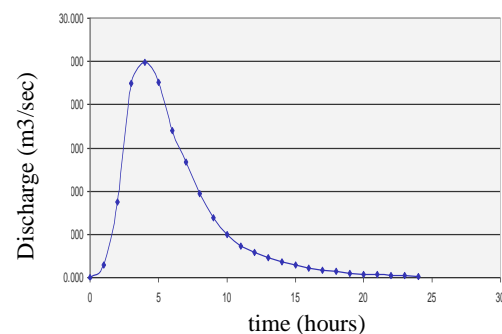


Fig 3. The unit hydrograph Synthetic of Sekanak sub-basin

Once the entire value discharge curve for each interval time is unknown, enter these value in the table below to get the value of runoff that occurs on an hourly basis with certain variations in rainfall. Unit hydrograph of Sekanak Sub-basin can be seen figure 3.

G. Land free of Sekanak river

Table 6. Kapten A Rivai road till the estuary of Sekanak river

Picture Code	Amount of Structure		Description
	Left of River	Right of River	
C-3-4-02	0	1	1-2
C-3-2-22	4	4	2-3
C-3-2-25	6	2	3-4
C-3-2-18	1	6	4-5
C-3-2-14	4	20	5-6
Total	48		

Note:

- 1-2: Jl. Kapten A Rivai – Jl. Letnan Mukmin
- 2-3: Jl. Letnan Mukmin – Jl. Letnan Jaimas
- 3-4: Jl. Letnan Jaimas – Jl. SW Subekti
- 4-5: Jl. SW Subekti – Lrg. Kosib II
- 5-6: Lrg. Kosib II – The estuary of Sekanak River

H. Condition, Land used and Structure

For condition, land used and structure on figure 4 belows

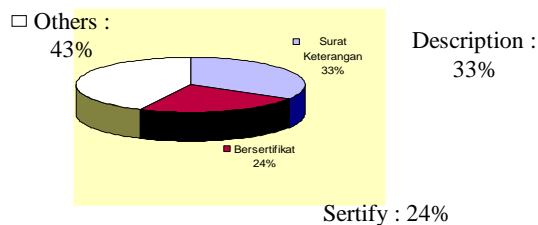


Figure 4 Land Status

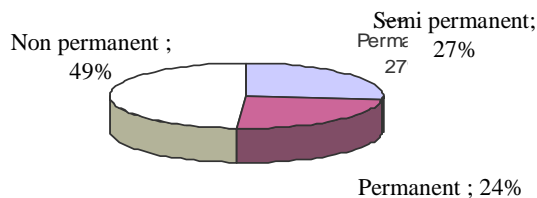


Fig 5 Structure Condition

IV. CONCLUSIONS

Cases like mentioned above also occur in Sekanak sub-basin, so it is necessary to study the drainage network performance evaluation system based on the concept of sustainable drainage based on community participation. Drainage system performance can be evaluated from the technical aspects as well as non-technical. One technical aspect is the frequency analysis to look at the picture unit hydrograph. Analytical results from this study may be that the

maximum flow of 25 m³/sec at peak hours at the time of 4.8 hours and then slowly starting to go down at a time to 24 hours.

Condition of land used and structure, only 24% structure has certify and only 24% is permanent structure. This is problem for city government to resolve flooding area, so we need the exact data especially rainfall data.

Social construction problems can be solved by Public Participation approach, with the Government acting on the principle of fair, development activities carried out with transparency and attention to the needs of the community.

Social problems occurring both at the pre-construction stage (land acquisition and resettlement), construction (procurement and mobilization of labor, mobilization of heavy equipment and construction materials and equipment operations) and post-construction can be anticipated by social engineering forming between Other identification of community character and institutions, socialization, public consultation, community gathering and community empowerment.

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